

International Journal of Engineering & Technology

Website: www.sciencepubco.com/index.php/IJET

Research paper



Flood Frequency Analysis at the Downstream of Sg. Perak River Basin using Annual Maximum Flow Discharge Data

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Abstract

The recent flood event occurred in 2014 had caused disaster in Perak and Sungai Perak is the main river of Perak which is a major natural drainage system within the state. The aim of this paper is to determine the expected discharge to return period downstream for Sg. Perak River Basin in Perak by using annual maximum flow data. Flood frequency analysis is a technique to assume the flow values corresponding to specific return periods or probabilities along the river at a different site. The method involves the observed annual maximum flow discharge data to calculate statistical information such as standard deviations, mean, sum, skewness and recurrence intervals. The flood frequency analysis for Sg. Perak River Basin was used Log Pearson Type-III probability distribution method. The annual maximum peak flow series data varying over period 1961 to 2016. The probability distribution function was applied to return periods (T) where T values are 2years, 5years, 10years, 25years, 50years, and 100years generally used in flood forecasting. Flood frequency analysis shows that Sg. Perak at Jambatan Iskandar much higher inflow discharge which is 3714.45m³/s at the 100years return period compare to Sg. Plus at Kg Lintang and Sg. Kinta at Weir G. With this, the 100years peak flow at Sg Perak river mouth is estimated to be in the range of 4,000 m3/s. Overall, the analysis relates the expected flow discharge to return period for all tributaries of Sg. Perak River Basin.

Keywords: Flood frequency analysis; maximum peak flow; discharge; return period.

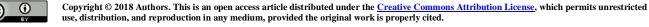
1. Introduction

Flood disasters are among the world is most common, damaging types of disaster and unexpected natural events causing misfortune of lives and harm to public property. In fact, it becomes the most important natural disaster in term of the population affected, frequency, financial cost and the disruption of socio-economic activities. [1] Different states in Malaysia, the flood has become a common disaster because it happened almost every year. The northeast monsoon usually happened between November and February caused the heavy rainfall as much as 610mm in 24 hours in ultimate cases primarily in the east coast of Peninsular Malaysia and to Sabah and Sarawak. Usually, flood happened from November to January when the northeast monsoon leads a large volume of runoff to the relatively large catchment areas. [2-3] Furthermore, climate change and variability have caused significant impacts on the hydrological cycle (flood and drought) as well as affecting the overall level of water availability in an urban city. [4-7] Flood frequency analysis is the most explicit method for determine the design flood which is very important part in the floodplain management, development and planning controls, and in the design of hydraulic structures. Usually, In Malaysia, a 100years ARI has been used for designing hydraulic structures. [9] In reducing the disaster risk and to effectively protect people, goods and infrastructures, is important to appraise and map the natural

hazard on the element of any natural disaster using Flood frequency analysis technique.

The purpose of using flood frequency analysis is to connect flood intensity with its probability of exceedance which represent the discharge and return period respectively. This information is important for divergent operational applications such as prevention of flood or civil engineering design including dams, dykes, any construction near a river. [9] The heavy and continuous rainfall will exceeding the absorptive capacity of the soil, and the flow capacity of the streams caused high flow exceeding danger levels and entering in floodplains. This event will causes damage to property and life in different parts of the catchment. [12] Previous studies have also been done to estimate flood based on catchment characteristics and statistical analysis by using at least 30 to 40 years of records data. The short data records and also missing data then regional flood frequency curves together provide consistent estimates of floods. [12] Moreover, the water availability, catchment characteristics and also possible extreme hydrological conditions like floods and droughts at different locations of the river system may be shown through the flood frequency analysis. [9] The flood frequency analysis was using the observed annual maximum flood data from gauging station to measure flood magnitude. [10] Statistical distribution method is needed for the purpose of when a long period of recorded flood data required [11].

The Log-Pearson Type III distribution is a statistical technique to assume the design flood for a different site of the river using the fitting frequency distribution data. The frequency distribution will



be established when the statistical information is calculated for the river site. The different sizes of probabilities of floods can be extracted from the curve. The advantage of this technique is that extrapolation can be formed using the value from the events with return periods exceed the observed flood events. The Federal Agencies in the United States also using this technique. [13] The purpose of the paper is to determine the expected discharge to return period downstream for Sg. Perak River Basin in Perak by using annual maximum flow data.

2. Study Area

Sg. Perak River is the main river in Perak which covers a catchment area of 3504 km² and river length approximately 400 km, flowing from Hulu Sg. Perak at upper of Temengor catchment through Bagan Datoh until Strait of Malacca. Sg. Perak is the second largest river in Peninsular Malaysia after Sg. Pahang. Figure 1 shows the overall catchment of Sungai Perak river basin. The annual rainfall is in the range of 2400 mm to 3200 mm at the northeast part of the basin, particularly in the upper Temenggor Dam catchment. At the south-eastern part of the basin, the annual rainfall increases between 2,600 mm and 3,400 mm. The average annual rainfall for the whole basin is about 2,300 mm. (IRBM, 2010) Figure 1 shows the Sg Perak Basin Catchment Delineation for Rainfall-Runoff Model.



Fig. 1: Sg Perak Basin Catchment Delineation for Rainfall-Runoff Model (source: G&P, 2010).

3. Methodology

Flood frequency analysis is used to assume design floods for sites along a river using the observed annual maximum flow discharge data to calculate statistical information such as mean values, standard deviations, skewness, and recurrence intervals. Then, it used to construct frequency distributions, which are graphs and tables that tell the probability of different discharges as a function of recurrence interval or exceed probability. Flood frequency distributions are divided into four (4) method. Four of the common methods are:

- a. Normal Distribution
- b. Log-Normal Distribution
- c. Gumbel Distribution
- d. Log-Pearson Type III Distribution

The method of data collecting is using Tideda where the real data through the installation of standard meteorological stations which belongs to government agencies and others agencies related. [8] Hydrological data of Malaysia is supports in both manual and automatic system by the Department of Irrigation and Drainage (DID) and other government agencies such as Tenaga Nasional Berhad (TNB) and other agencies.

The available manual and auto logger type streamflow stations for DID in Perak, total up to 34 units of stations. The rainfall stations

for TNB are two units of stations. Figure 2 shows the location of streamflow stations accordingly to its coordinates.

By reviewing the stream flow data collection in Sg. Perak Hydroelectric Scheme, a decision regarding the selection of appropriate and suitable rainfall stations that can be used for this study was done. The available longest records data should be at least up to 10 years. Table 1 shows the selected streamflow stations in this study and figure 3 shows the Analysis Technique on Flood Frequency Analysis using Log-Pearson Type III Distribution. For DID station, the stream flow recording stations available in the study from the year open up to the year 2016. There were two (2) TNB's stream flow recording stations available in the study area. However, both stations were closed in the year 1982 and 1996.



Fig. 2: Location of available streamflow stations located in the State of Perak.

Table 1: List of DID and TNB	Stream flow Station
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No.	Station No.	Station Name	Owner	
1.	3913458	Sg. Sungkai at Sungkai	DID	
2.	4012401	Tg. Bidor at Malayan Bidor	DID	
3.	4111455	Sg. Btg. Padang at Tg. Keramat	DID	
4.	4212467	Sg. Chendering at Bt. 32 Jalan Tapah	DID	
5.	4310401	Sg. Kinta at Weir G at Tg. Tualang	DID	
6.	4311464	Sg. Kampar at Kg. Lanjut	DID	
7.	4511468	Sg. Raia at Keramat Pulai	DID	
8.	4610466	Sg. Pari at Jln. Salibin	DID	
9.	4611463	Sg. Kinta at Tg. Rambutan	DID	
10.	4809443	Sg. Perak at Jambatan Iskandar	DID	
11.	4911445	Sg. Pelus at Kg. Lintang	DID	
12.	5610401	Sg. Rui at Jambatan Jalan Raya	DID	
13.	6011	Sg. Perak at Kuala Temain	TNB	
14.	6029	Sg. Piah at Kuala Chenderoh	TNB	

Obtain daily stream flow data: Calculate the average, minimum and maximum discharge for each

Vear.	acn
5	_
Rank the data from largest discharge,Q to smallest discharge and number each stream flow value	e from
1 to n.	
Calculate each maximum stream flow using the excel formula {log (Q)}	
₹۶	
Calculate the Average Maximum Q and the average of the log (Q)	
₹۶	
Calculate the {(log Q - average (logQ))^2} and {(log Q - average (logQ))^3}	
Calculate the return period (Tr) for each discharge using excel formula {(n+1)/m}. Where n= number of values in the dataset and m= the rank	the
Calculate the exceedence probability of each discharge using excel formula {=1/Return period	dor
1/Tr}	0.01
₹5	
Calculate the Sum for the $\{(\log Q - average (\log Q))^2\}$ and $\{(\log Q - average (\log Q))^3\}$	
V	
Calculate the variance, standard deviation, and skew coefficient	
V	
Calculate the k value : Using the frequency factor table	
۲۶	
Using the general equation, list the discharges associated each recurrence interval	
Create table of Discharge values found using the log-Pearson analysis	
V	
Create plot	

Fig. 3: Analysis Technique on Flood Frequency Analysis using Log-Pearson Type III Distribution.

4. Results and Discussion

Flood frequency analysis was also carried out at all selected stream flows stations including Sg Pelus at Kg Lintang and Sg Kinta at Weir G, which are the major tributaries of Sg Perak. Table 2 shows the summary of the flood frequency analysis results.

Table 2: List of	DID and	l TNB Stre	am flow	Station

Station			Peak Flows (m3/s)				
No.	Station Name	Years	Q 2	Q 5	Q 25	Q 50	Q 100
3913458	0 0	1961-	52.14	85.27	131.59	148.92	165.07
	Sungkai	2016					
4012401	Tg. Bidor at	1981-	55.74	100.56	176.99	210.31	243.81
	Malayan Bidor	2016					
4111455	Sg. Btg.	1961-	73.71	104.53	140.34	152.17	162.54
	Padang at Tg.	2016					
	Keramat						
4212467	Sg. Chender- ing	1971-	27.19	41.45	59.35	65.57	71.13
	at Bt. 32	2016					
	Jalan Tapah						
4310401	Sg. Kinta at Weir	1977-	276.36	411.59	643.93	750.18	862.53
	G at Tg.	2016					
	Tualang						
4311464	Sg. Kampar at	1961-	32.89	67.41	147.41	191.69	242.88
	Kg. Lanjut	2016					
4511468	Sg. Raia at	1978-	28.31	65.10	132.11	160.72	188.63
	Keramat Pulai	2016					
4610466	Sg. Pari at Jln.	1961-	59.60	106.47	174.38	199.92	223.54
	Salibin	2016					
4611463	Sg. Kinta at Tg.	1961-	50.94	92.62	172.43	210.93	252.12
	Rambutan	2016					
4809443	Sg. Perak at	1961-	584.49	1047.74	2191.22	2875.43	3714.45
	Jambatan	2016					
	Iskandar						
4911445	Sg. Pelus at Kg.	1961-	95.21	153.88	235.08	265.31	293.40
	Lintang	2016					
5610401	Sg. Rui at	1987-	31.14	59.65	115.20	142.08	170.81
	Jambatan Jalan	2016					
	Raya						
6011	Sg. Perak at	1966-	426.03	963.56	2084.19	2630.35	3207.85
	Kuala Temain	1981					
6029	Sg. Piah at Kuala	1981-	436.42	965.36	1969.27	2420.61	2877.59
	Chen-deroh	1995					

The 2years and 100years peak flow for Sg Perak at Jambatan Iskandar are 584.49 m³/s and 3714.45 m³/s respectively. Sg Pelus is the largest tributary that joins Sg Perak between the Chenderoh Dam and Sg Kinta confluence. Large parts of the Pelus catchment is still dominated by forest and plantation tree crops. The 2years and 100years ARI peak flow for Sg Pelus at Kg Lintang are 95.21 m³/s and 293.40 m³/s respectively.

For Sg Kinta at Weir G, the 100years peak flow is computed to be 862.53 m^3 /s using frequency analysis in this Study. The remarkable increase in 100years in this study is mainly due to two recent high storm events which occurred in October 2008 (746 m^3 /s) and January 2009 (767 m^3 /s). The increase in peak flow could also be attributed to continuous land development activities, which have resulted in higher flood flow.

The major tributary, Sg Bidor with the catchment area of 1,787 km² is located south of the Sg Kinta catchment. The 100years peak flow for Sg Bidor catchment could be estimated to be of the same magnitude as Sg Kinta at Tg Rambutan based on the similar catchment size. The lateral catchments along Sg Perak at the downstream of Chenderoh Dam have a total combined catchment area of 2,596 km². Using the simple area proportional method transposed from 100years of Sg Kinta at Weir G, the combined 100years peak flow for the lateral catchment is about 1,293 m³/s.

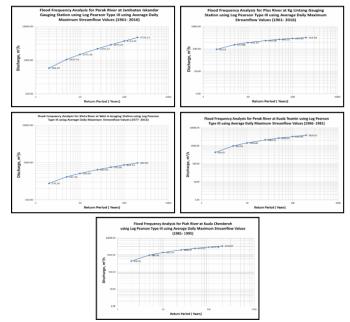


Fig. 4: Flood frequency analysis graph using Log-Pearson Type III distribution for selected stations.

5. Conclusion

The results for flood frequency analysis shows that Sg. Perak at Jambatan Iskandar much higher inflow discharge which is $3714.45m^3$ /s at the 100years, return period compares to Sg. Plus at Kg Lintang 293.40 m³/s and Sg. Kinta at Weir G 862.53 m³/s. With this, the 100years peak flow at Sg Perak river mouth is estimated to be in the range of 4,000 m³/s. Overall, the analysis relates the expected flow discharge to return period for all tributaries of Sg. Perak River Basin.

The Log-Pearson Type III distribution shows the values of expected discharges from the river at different recurrence intervals based on the available historical record. It very important for designing structures in or near the river which may be affected by floods and also to protect against the largest expected event.

Acknowledgement

The authors like to acknowledge the International Centre for Water Hazard and Risk Management (ICHARM), Tenaga Nasional Berhad Research (TNBR) for their support and collaboration in making this project successful. A many thanks to a research team from the Centre for Sustainable Technology and Environment Group (STEG), Universiti Tenaga Nasional (UNITEN) give full commitment to conduct this research.

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